

2. CHS KT-joint (K)

The design of axial resistance of the CHS KT-joint is performed according to prEN 1993-1-8(2021), see Figure 2.1. The exact description and material properties of each member are given. The classification of hollow section truss-type joints as T, Y, X or K gap joints (which includes N-type joints) should be based on the method of force transfer in a joint but not on its physical appearance. Since there is no internal force in member 3, the KT joint considers as the K-joint.

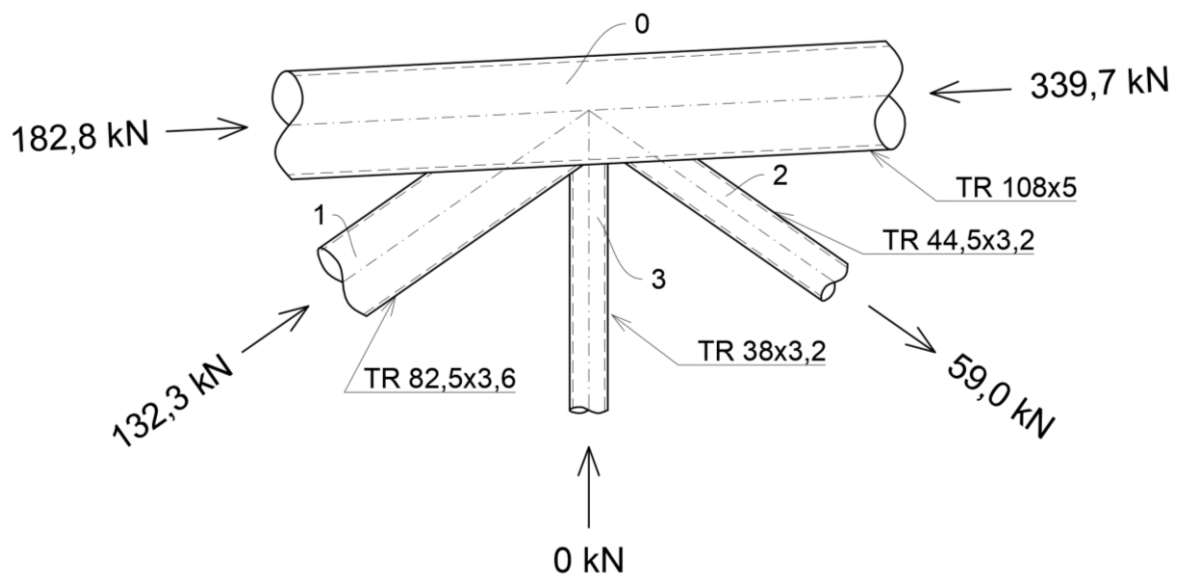


Figure 2.1 Joint geometry, internal forces

2.1. Symbols

d_i	is an overall diameter of CHS member i ($i=0,1,2$ or 3)
e	is an eccentricity of a joint
$f_{y,i}$	is a yield strength of member i ($i=0,1,2$ or 3)
A_i	is a cross sectional area of member i ($i=0,1,2$ or 3)
$W_{el,i}$	is an elastic section modulus of member i ($i=0,1,2$ or 3)
θ_i	is an included angle between brace member i and the chord ($i=1,2$ or 3)
β	is a ratio of the mean diameter or width of brace members, to that of the chord
γ	is a ratio of a chord width or diameter to twice its wall thickness
Q_f	is a chord stress factor

n	is a chord stress parameter
C_f	is a material factor
$M_{ip,i,Ed}$	is a design in-plane internal moment in member i ($i=0,1,2$ or 3)
$M_{op,i,Ed}$	is a design out-of-plane internal moment in member i ($i = 0, 1, 2$ or 3)
$N_{i,Ed}$	is a design internal axial force in member i ($i=0,1,2$ or 3)
$N_{i,Rd}$	is a design resistance of a joint expressed in terms of the internal axial force in member i ($i = 0, 1, 2$ or 3)

2.2. Design forces

$N_{0,Ed} = -339\,700\text{ N}$	$N_{1,Ed} = -132\,300\text{ N}$
$M_{ip,0,Ed} = 0,0\text{ Nmm}$	$M_{ip,1,Ed} = 0,0\text{ Nmm}$
$M_{op,0,Ed} = 0,0\text{ Nmm}$	$M_{op,1,Ed} = 0,0\text{ Nmm}$
$N_{2,Ed} = 59\,000\text{ N}$	$N_{3,Ed} = 0,0\text{ N}$
$M_{ip,2,Ed} = 0,0\text{ Nmm}$	$M_{ip,3,Ed} = 0,0\text{ Nmm}$
$M_{op,2,Ed} = 0,0\text{ Nmm}$	$M_{op,3,Ed} = 0,0\text{ Nmm}$

2.3. Material

$f_{y,0} = 355,0\text{ MPa}$	$f_{y,1} = 355,0\text{ MPa}$	$f_{y,2} = 355,0\text{ MPa}$
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2.4. Geometry of sections

$A_0 = 1\,618\text{ mm}^2$
$W_{el,0} = 39,8 \cdot 10^3\text{ mm}^3$

2.5. Range of validity

$$0,2 \leq \frac{d_1}{d_0} = \frac{82,5}{108,0} = 0,76 \leq 1,0$$

$$0,2 \leq \frac{d_2}{d_0} = \frac{44,5}{108,0} = 0,41 \leq 1,0$$

$$\frac{d_1}{t_1} = \frac{82,5}{3,6} = 22,92 \leq 50$$

$$\frac{d_2}{t_2} = \frac{44,5}{3,2} = 13,91 \leq 50$$

$$10 \leq \frac{d_0}{t_0} = \frac{108,0}{5,0} = 21,60 \leq 50$$

$$-0,55 \leq \frac{e}{d_0} = \frac{0,0}{108,0} = 0,0 \leq 0,25$$

$$g = 41,6 \text{ mm} \leq t_1 + t_2 = 3,2 + 3,6 = 6,8 \text{ mm}$$

$$t_1 = 3,6 \text{ mm} \leq t_0 = 5,0 \text{ mm}$$

$$t_2 = 3,2 \text{ mm} \leq t_0 = 5,0 \text{ mm}$$

$$\theta_1 = 32,1^\circ \leq 30^\circ$$

$$\theta_2 = 37,9^\circ \leq 30^\circ$$

2.6. Geometric ratios

$$\gamma = \frac{d_0}{2 \cdot t_0} = \frac{108,0}{2 \cdot 5,0} = 10,80$$

$$\beta = \frac{d_1 + d_2}{2 \cdot d_0} = \frac{82,5 + 44,5}{2 \cdot 108,0} = 0,58$$

2.7. Design of axial resistance of the joint

$$n = \frac{N_{o,Ed}}{A_0 \cdot f_{y,o}} + \sqrt{\left(\frac{M_{ip,0,Ed}}{W_{el,0} \cdot f_{y,o}}\right)^2 + \left(\frac{M_{op,0,Ed}}{W_{el,0} \cdot f_{y,o}}\right)^2}$$

$$= \frac{-339\,700}{1\,618 \cdot 355} + \sqrt{\left(\frac{0,0}{39,8 \cdot 10^3 \cdot 355}\right)^2 + \left(\frac{0,0}{39,8 \cdot 10^3 \cdot 355}\right)^2} = -0,59$$

C_1	
$n < 0$ (compression)	$n \geq 0$ (tension)
$C_1 = 0,45 - 0,25 \cdot \beta$	$C_1 = 0,20$

Table 2.1 Exponent for a chord stress factor

$$C_1 = 0,45 - 0,25 \cdot \beta = 0,45 - 0,25 \cdot 0,35 = 0,31$$

$$Q_f = (1 - |n|)^{C_1} = (1 - |-0,59|)^{0,31} = 0,76$$

C_f	
$f_y \leq 355 \text{ MPa}$	$C_f = 1,0$
$355 \text{ MPa} < f_y \leq 460 \text{ MPa}$	$C_f = 0,9$
$460 \text{ MPa} < f_y \leq 700 \text{ MPa}$	$C_f = 0,8$

Tab. 2.2 Material factors to resistance

$$C_f = 1,0$$

2.7.1. Chord face failure

$$N_{1,Rd} = \frac{C_f \cdot \frac{f_{y0} \cdot t_0^2}{\sin \theta_1} \cdot (1,65 + 13,2 \cdot \beta^{1,6}) \cdot \gamma^{0,3} \cdot \left[1 + \frac{1}{1,2 + \left(\frac{g}{t_0}\right)^{0,8}} \right] \cdot Q_f}{\gamma_{M5}}$$

$$= \frac{1,0 \cdot \frac{355,0 \cdot 5,0^2}{\sin 32,1} \cdot (1,65 + 13,2 \cdot 0,58^{1,6}) \cdot 10,80^{0,3} \cdot \left[1 + \frac{1}{1,2 + \left(\frac{41,6}{5,0}\right)^{0,8}} \right] \cdot 0,76}{1,25}$$

$$= 171\,065 \text{ N}$$

$$N_{2,Rd} = \frac{\sin \theta_1}{\sin \theta_2} \cdot N_{1,Rd} = \frac{\sin 32,1}{\sin 37,9} \cdot 171\,065 = 147\,982 \text{ N}$$

2.7.2. Punching shear failure

Validity of the punching shear check:

$$d_1 = 82,5 \text{ mm} \leq d_0 - 2 \cdot t_0 = 108,0 - 2 \cdot 5,0 = 98,0 \text{ mm}$$

$$N_{1,Rd} = \frac{C_f \cdot \frac{f_{y0}}{\sqrt{3}} \cdot t_0 \cdot \pi \cdot d_1 \cdot \frac{1 + \sin \theta_1}{2 \cdot \sin^2 \theta_1}}{\gamma_{M5}} = \frac{1,0 \cdot \frac{355,0}{\sqrt{3}} \cdot 5,0 \cdot \pi \cdot 82,5 \cdot \frac{1 + \sin 32,1}{2 \cdot \sin^2 32,1}}{1,25}$$

$$= 576\,167 \text{ N}$$

$$N_{2,Rd} = \frac{C_f \cdot \frac{f_{y0}}{\sqrt{3}} \cdot t_0 \cdot \pi \cdot d_2 \cdot \frac{1 + \sin \theta_1}{2 \cdot \sin^2 \theta_1}}{\gamma_{M5}} = \frac{1,0 \cdot \frac{355,0}{\sqrt{3}} \cdot 5,0 \cdot \pi \cdot 44,5 \cdot \frac{1 + \sin 37,9}{2 \cdot \sin^2 37,9}}{1,25}$$

$$= 245\,158 \text{ N}$$

Design resistance of the joint expressed in terms of the internal axial force in member 1 is

$$N_{1,Ed} = 39\,281 \text{ N} \text{ and in member 2 is } N_{2,Rd} = 147\,982 \text{ N.}$$

2.8. Design check

$$\frac{N_{1,Ed}}{N_{1,Rd}} = \frac{132\,300}{171\,065} = 0,77 \leq 1,0$$

$$\frac{N_{2,Ed}}{N_{2,Rd}} = \frac{59\,000}{147\,982} = 0,40 \leq 1,0$$

It is satisfied.